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equal to the maximum dynamic response of the element or component under consideration.

EXCEPTION: Elements of seismically isolated structures and nonstructural components or portions designed to resist seismic forces and displacements as prescribed in Chapter 12 or 13 as appropriate.

17.2.6.2 Components Crossing the Isolation Interface. Elements of seismically isolated structures and nonstructural components, or portions thereof, that cross the isolation interface shall be designed to withstand the total maximum displacement.

17.2.6.3 Components below the Isolation Interface. Elements of seismically isolated structure and nonstructural components, or portions thereof, that are below the isolation interface shall be designed and constructed in accordance with the requirements of Sections 12.1 and 13.

17.3 GROUND MOTION FOR ISOLATED SYSTEMS

17.3.1 Design Spectra. The site-specific ground motion procedures set forth in Chapter 21 are permitted to be used to determine ground motions for any structure. For structures on Site Class F sites, site response analysis shall be performed in accordance with Section 21.1. For seismically isolated structures on sites with S_1 greater than or equal to 0.6, a ground motion hazard analysis shall be performed in accordance with Section 21.2. Structures that do not require or use site-specific ground motion procedures shall be analyzed using the design spectrum for the design earthquake developed in accordance with Section 11.4.5.

A design spectrum shall be constructed for the maximum considered earthquake. This design spectrum for the maximum considered earthquake shall not be taken as less than 1.5 times the design spectrum for the design earthquake.

17.3.2 Ground Motion Histories. Where response history procedures are used, ground motions shall consist of pairs of appropriate horizontal ground motion acceleration components that shall be selected and scaled from individual recorded events. Appropriate ground motions shall be selected from events having magnitudes, fault distance, and source mechanisms that are consistent with those that control the maximum considered earthquake. Where the required number of recorded ground motion pairs are not available, appropriate simulated ground motion pairs shall be used to make up the total number required. For each pair of horizontal ground-motion components, a square root of the sum of the squares (SRSS) spectrum shall be constructed by taking the SRSS of the 5 percent damped response spectra for the scaled components (where an identical scale factor is applied to both components of a pair). Each pair of motions shall be scaled such that for each period between $0.5T_D$ and $1.25T_M$ (where T_D and T_M are defined in Section 17.5.3) the average of the SRSS spectra from all horizontal component pairs does not fall below 1.3 times the corresponding ordinate of the design response spectrum, determined in accordance with Section 17.3.1, by more than 10 percent.

17.4 ANALYSIS PROCEDURE SELECTION

Seismically isolated structures except those defined in Section 17.4.1 shall be designed using the dynamic procedures of Section 17.6.

17.4.1 Equivalent Lateral Force Procedure. The equivalent lateral force procedure of Section 17.5 is permitted to be used for design of a seismically isolated structure provided that:

1. The structure is located at a site with S_1 less than $0.60g$.
2. The structure is located on a site Class A, B, C, or D.

3. The structure above the isolation interface is less than or equal to four stories or 65 ft (19.8 m) in height.
4. The effective period of the isolated structure at the maximum displacement, T_M , is less than or equal to 3.0 s.
5. The effective period of the isolated structure at the design displacement, T_D , is greater than three times the elastic, fixed-base period of the structure above the isolation system as determined by Eq. 12.8-7 or 12.8-8.
6. The structure above the isolation system is of regular configuration.
7. The isolation system meets all of the following criteria:
 - a. The effective stiffness of the isolation system at the design displacement is greater than one-third of the effective stiffness at 20 percent of the design displacement.
 - b. The isolation system is capable of producing a restoring force as specified in Section 17.2.4.4.
 - c. The isolation system does not limit maximum considered earthquake displacement to less than the total maximum displacement.

17.4.2 Dynamic Procedures. The dynamic procedures of Section 17.6 are permitted to be used as specified in this section.

17.4.2.1 Response-Spectrum Procedure. Response spectrum analysis shall not be used for design of a seismically isolated structure unless:

1. The structure is located on a site Class A, B, C, or D.
2. The isolation system meets the criteria of Item 7 of Section 17.4.1.

17.4.2.2 Response History Procedure. The response history procedure is permitted for design of any seismically isolated structure and shall be used for design of all seismically isolated structures not meeting the criteria of Section 17.4.2.1.

17.5 EQUIVALENT LATERAL FORCE PROCEDURE

17.5.1 General. Where the equivalent lateral force procedure is used to design seismically isolated structures, the requirements of this section shall apply.

17.5.2 Deformation Characteristics of the Isolation System. Minimum lateral earthquake design displacements and forces on seismically isolated structures shall be based on the deformation characteristics of the isolation system. The deformation characteristics of the isolation system shall explicitly include the effects of the wind-restraint system if such a system is used to meet the design requirements of this standard. The deformation characteristics of the isolation system shall be based on properly substantiated tests performed in accordance with Section 17.8.

17.5.3 Minimum Lateral Displacements.

17.5.3.1 Design Displacement. The isolation system shall be designed and constructed to withstand minimum lateral earthquake displacements, D_D , that act in the direction of each of the main horizontal axes of the structure in accordance with the following:

$$D_D = \frac{g S_{D1} T_D}{4\pi^2 B_D} \quad (17.5-1)$$



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